$\mathbf{D}_{\mathrm{ata}}$

 $N_{\rm eeds}$

Analysis



Scoping Study











US 60 Spottsville Bridge (051B00015N) over Green River

Henderson County

Item No. **02 - 1080.00**

Prepared by

KYTC District 2 Planning

August 2013



	I. PRELIMIN <i>E</i>	ARY PROJECT INFORMA	TION				
County:	Henderson	Item No.:	2-1080.00				
Route Number(s):	US 60	Road Name:	US 60				
Program No.:	8675901D	UPN: FD52	051 0060 019-020				
Federal Project No.:		Type of Work:	Bridge Replacement				
	Plan Project Description:						
			(SR39) 051B00015N (12CCR)				
Beginning MP:	: 19.236	Ending MP: 19.444	Project Length: 0.208				
Functional Class.:	☐ Urban ☑ Rural	State Class.:	☐ Primary ☐ Secondary				
	Arterial 🔻	Route is on:	□ NHS ☑ NN □ Ext Wt				
MPO Area: Evansville/H	lenderson 🔻	Truck Class.:	AAA 💌				
In TIP: ☑ Yes ☐ I		% Trucks:	5.70%				
ADT (current):	3240 (2012)	Terrain: Roll					
		/ Controlled ☐ Partial	Spacing:				
Existing Bike Accomm			: Sidewalk				
	35 mph	☐ 55 mph	☐ Other (Specify):				
KYTC Guidelines Preli		45 MPH Proposed Design Speed					
	minum, Ducca co						
Roadway Data:	EXISTING	COMMON GEOMETRIC PRACTICES*					
No. of Lanes	<u>2</u>	2	Existing Rdwy. Plans available?				
Lane Width	<u>-</u> 9 ft	<u>=</u> <u>12 ft</u>	✓ Yes □ No				
Shoulder Width	0.3 ft	8 ft	Year of Plans: 1933				
Max. Superelevation**	-13.11		Traffic Forecast Requested				
Minimum Radius**		600 ft	Date Requested: 5/23/2013				
Maximum Grade		<u>6%</u>					
Minimum Sight Dist.		<u>60 ft</u>	☐ Mapping/Survey Requested Date Requested:				
Sidewalk Width(urban)		0010					
Clear-zone***			Type:				
Clear-zone*** Project Notes/Design Ex	reantions?						
_	-	esign of Highways and Streets, ***AASH	ITO's Roadside Design Guide				
Bridge No. 051B0001	(Bridge #1)	(Bridge #2)					
Sufficiency Rating	38.9	[5110]5 11-]	Existing Geotech data available?				
Total Length	1103 ft		✓ Yes No				
Width, curb to curb	19.8 ft						
Span Lengths	359.9 ft max		Detour Length(s): 10.0 miles				
			Detour Length(s). 10.0 miles				
Year Built	1930						
Posted Weight Limit	<u>33T</u>		*If more than two bridges are located on				
Structurally Deficient? Functionally Obsolete?	<u>Yes</u>		the project, include additions sheets.				
runctionally obsoleter	162		, , ,				

1

Existing Bridge Type	Steel truss w concrete								
II. PROJECT PURPOSE AND NEED									
A. Legislation									
Item # 2-1080.00 on th	Funding	Phase	Year	Amount					
		BRO	D	2013	\$1,500,000				
		BRO	R	2014	\$250,000				
		BRO	U	2014	\$250,000				
		BRO	С	2015	\$25,000,000				

B. Project Status

This project was UPL# 2 051 B0060 50.00. It was first requested by EUTS(EMPO) in 2004. Design funds in the amount of \$900,000 were authorized on March 4th, 2013.

C. System Linkage

US 60 is a Rural Minor Arterial linking several small communities to the city of Henderson to the west and to the city of Owensboro to the east. This project will not change the functional classification.

D. Modal Interrelationships

This bridge interacts with barge traffic on the Green River. River traffic has collided with the bridge in the past resulting in the bridge being closed for repairs. This segment of US 60 carries 5.7% truck traffic (2010). The bridge is posted with a 33T weight limit currently. The route is designated as a truck route and is inclusive of a US bike trail.

E. Social Demands & Economic Development

This section of US 60 runs from the north end of Henderson to the north end of Owensboro and is one of only 2 bridges that cross the Green River between the 2 cities. US 60 also connects several other smaller towns in the area including Spottsville, Reed, and Stanley. This segment of US 60 is on the Scenic Byway system- Blues to Bluegrass- W. C. Handy Blues Trail. J. J. Audubon State Park is located between the bridge and Henderson. Ben Hawes State Park is located between the bridge and Owensboro.

F. Transportation Demand

The average daily traffic across this bridge was 3240 in 2012 with approximately 6% truck traffic. A 2005 MPO freight survey identified this bridge as an impediment to freight mobility.

2

U DDOUGOT DUDDOGG AND NIGED ()
II. PROJECT PURPOSE AND NEED (cont.)
G. Capacity
This project will not affect capacity.
H. Safety
The existing bridge over Green River at Spottsville is substandard. The bridge is a narrow two-lane bridge that does not
adequately handle the truck traffic on this road. 6 collisions were reported on the bridge in the 3 yr period between
1/1/2010 and 1/1/2013. One was an animal collision and the remaining 5 were side swipe collisions involving 2 vehicles
moving in opposite directions.
I. Roadway Deficiencies
19.8 ft curb-to-curb is narrow for the large volume of truck traffic using this bridge. KYTC's Common Geometric
Practices recommends 24 ft pavement width and 8 ft shoulders for a rural arterial roadway.
Draft Purpose and Need Statement:
Need: Bridge replacement. (051B00015N) over Green River is a narrow 2 lane bridge which is structurally deficient. It
has a sufficiency rating of 38.9. Multiple collisions have been reported on this bridge.

Purpose: Improve safety, increase mobility, and address structural and roadway deficiencies on the bridge.

3

8/8/2013

	III.	PRELIMINA	RY ENVIRONM	ENTAL OVE	RVIEW	
A. Air Quality Project is in: STIP Pg.#: 50	☑ Attainment area	□Nonattair	nment or Maintena TIP Pg.#:	nce Area	☐ PM 2.5 County	
Jiii 1 g.m. 30			8			
	/Historic Resource logical or Historic Reso		nt			
_	s steel truss built in 1 eology: dependent u		_	•) with documentation will likely be new alignment.	
C. Threatened a	and Endangered S	pecies				
			nn extensive Mar	mmal and Mu	issel Biological Assessment will likely	y be
D. Hazardous I ☐ Potentially Cor	Materials ntaminated Sites are pr	esent 🖸	Potential Bridge o	or Structure Dei	molition	
Bridge must be ir	nspected by DEA Brid	dge Inspectors	s for lead-based	paint and asb	oestos.	
Are 401/404 Peri	ny apply: ☐ Waters of mits likely to be requ CE LON ☑ ACE N		area		☑ Navigable Waters of the US Impacts ☑ Wetlands ☐ Stream/Lake/Pond ☐ Special Use Waters	
USFWS National	Wetlands present as	well as the G	reen River.			
	anned noise sensitiv "Type I Project" accor	•	-		ect? ☑ Yes ☐ No atement Policy? ☐ Yes ☐ No	
Dependent on w	hether a new alignm	ent is chosen	•			
	ay apply: Low Inco			☑ Relocatio		
A possible new a noise for existing		aches have be	en mentioned. T	his would po	ssibly cause relocations and increas	ed
The following are p	or 6(f) Resources	☐ Sec	ction 4(f) Resource	s □ Sed	ction 6(f) Resources	
None known at thi	s time.					
Anticipated	d Environmental Do	cument:	CE Level 2		•	

IV. PROJECT SCOPING

This project is to replace a narrow 2 lane bridge with a suffiency rating of 38.9. The new bridge will remain 2 lanes. However, it has not been determined if it will be on existing or new alignment. If a new alignment is chosen, additional utility funds may be needed. This project will improve safety, mobility, and address structural and roadway deficiencies.

Current Estimate								
<u>Phase</u>	<u>Estimate</u>							
Planning								
Design	1,500,000							
R/W	250,000							
Utilities	250,000							
Const	25,000,000							
Total	27,000,000							











V. Summary

6

US 60 is the northern connection between Henderson and Owensboro. This project is to replace the narrow, structurally deficient bridge that crosses the Green River. If the design is on new alignment, an additional \$250,000 may be needed for Utilities.

VI. Tables and Exhibits





After Bridge (WB on 60)

After Bridge (EB on 60)



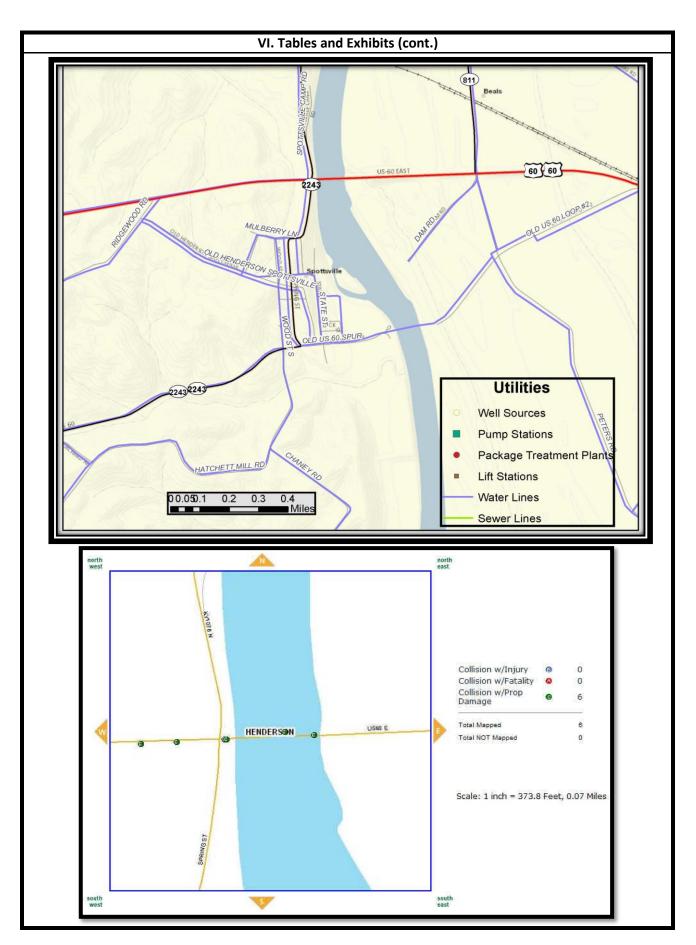


Just Before Bridge (WB on 60)

Before Bridge (WB on 60)



On Bridge (EB on 60)



Structure Invéntory and Appraisal Sheet (English Units)

Bridge Key: 6001 Agency ID: 051B00015N SR: 38.9 SD/FO: SD

Frequency 91:

IDENTIFICATION

21 Kentucky Facility Carried 7:

US-60

1 Mainline

FIPS 0000

Struc Num 8: 051B00015N Location 9:

.01 MI EAST OF JCT KY1078

Rte.(On/Under)5A: Route On Structure Rte. Signing Prefix 5B: 2 U.S. Numbered Rte. Number 5D:

00060

Directional Suffix 5E: 0 N/A (NBI) SHD District 2: District 2

% Responsibility: County Code 3;

Unknown Henderson (051)

19.340 mi Mile Post 11:

Feature Intersected 6: GREEN RIVER

Latitude 16: 37d 51' 45°

087d 24°41° Longitude 17:

Border Bridge Code 98: Unknown (P)

Border Bridge Number 99:

Level of Service 5C:

Place Code 4:

STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: 6

Number of Spans Main Unit 45: 2

Main Span Material/Design 43A/B:

3 Steel

10 Truss-Thru

Approach Span Material/Design 44A/B:

3 Steel

09 Truss-Deck

Deck Type 107:

1 Concrete-Cast-in-Place

Wearing Surface 108A:

4 Low Slump Concrete

Membrane 1088:

0 None

Deck Protection 108C: None

AGE AND SERVICE

Year Built 27;

1930

Year Reconstructed 106: 0

Type of Service on 42A: 1 Highway Type of Service under 42B: 5 Waterway

3.240

Lanes on 28A; 2 ADT 29:

Lanes Under 28B: 0

Truck ADT 109: 6 %

Detour Length 19: 10.0 mi Year of ADT 30: 2012

GEOMETRIC DATA

Length Max Span 48: 359.9 ft

Structure Length 49:

1,103.0 ft Curb/Sidewalk Width R 50B: 0,3 ft

Curb/Sdwlk Width £ 50A: 0.3 ft Width Curb to Curb 51:

Width Out to Out 52:

24.2 ft

Approach Roadway Width 32: 22.0 ft (w/ shoulders)

Median 33; 0 No median

Deck Area: 26,648.9 sq. ft Skew 34: 0.00 *

Vertical Clearance 10; 15.32 ft

Structure Flared 35: 0 No flare Horiz, Clearance 47: 19,83 ft

Minimum Vertical Clearance Over Bridge 53:

Minimum Vertical Underclearance Reference 54A:

N Feature not hwy or RR

Minimum Vertical Underclearance 54B:

0.0 ft

15.3 ft

Minimum Laterat Underclearance Reference R 55A:

N Feature not hwy or RR

Minimum Lateral Underclearance R 55:

0.0 ft a a n

Minimum Lateral Underclearance L 56:

INSPECTION

12 months Inspection Date 90:

3/28/2013 Next Inspection:

03/28/2014

FC Frequency 92A: 24 months FC Inspection Date 93A: 10/4/2011 Next FC Inspection: 10/4/2013

UW Frequency 928: 60 months UW Inspection Date 938: 5/25/2006 Next UW Inspection: 5/25/2011

SI Frequency 92C; NA SI Date 93C:

Element Frequency: 12 months Element Inspection Date: 03/28/2013 Next Elem. Insp. Due: 03/28/2014

CLASSIFICATION

0 Not a STRAHNET hwy Paratiel Structure 101:

3 On free road

No || bridge exists Temporary Structure 103:

Not Applicable (P)

5 Not eligible for NRHP

Direction of Traffic 102: 2 2-way traffic Highway System 104: -- 0 Not on NHS-

- NBIS Length 112: - - -

Long Enough

Functional Class 26: 06 Rural Minor Arterial 0 Not a STRAHNET hwy Historical Significance 37:

Toll Facility 20: Defense Hwy 110:

Owner 22; 01 State Highway Agency Custodian 21: 01 State Highway Agency

CONDITION

Sub 60: 5 Fair

Deck 58; 6 Satisfactory Culvert 62: N N/A (NBI)

Design Load 31:

Bridge Rail 36A:

Transition 368:

Total Cost 96:

Navigation Control 38:

Vertical Clearance 39:

Pier Protection 111:

Channel/Channel Protection 61:

Super 59; 4 Poor

7 Minor Damage

LOAD RATING AND POSTING

Operating Rating Method 63:

Inventory Rating Method 65: 1 LF Load Factor Inventory Rating 66: HS14.5

Operating Rating 64:

Posting 70:

5 At/Above Legal Loads

Posting status 41: P Posted for load

APPRAISAL

0 Substandard

2 M 13.5 (H 15)

0 Substandard

Approach Rail 36C: Approach Rail Ends 36D: Deck Geometry 68:

N Not applicable (NBI)

1 Meets Standards 1 Meets Standards 2 Intelerable - Replace

31 Repl-Load Capacity

Str. Evaluation 67: Underclearance, Vertical and Horizontal 69:

Waterway Adequacy 71: 8 Equal Desirable

Approach Alignment 72: 8 Equal Desirable Crit

8 Stable Above Footing Scour Critical 113:

PROPOSED IMPROVEMENTS

Bridge Cost 94: \$ 3,640,000 Roadway Cost 95: \$ 100,000

Year of Cost Estimate 97: 1999

\$ 3,739,000

Length of Improvement 76: Future ADT 114; Year of Future ADT 115:

Type of Work 75:

110.2 ft 4,827 2032

NAVIGATION DATA

1 Permit Required

79.4 ft

5 None, Re-Evaluate

Horizontal Clearance 40: Lift Bridge Vertical Clearance 116:

ELEMENT CONDITION STATE DATA

Str Unit	Elm/Env	Description	Units	Total Oty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5
1	22/1	P Conc Deck/Rigid Ov	(SF)	21,912	0 %	0	100 %	21,912	·0%	0	0 %	0	0 %	
1	11 0/1	R/Coлc Open Girder	(LF)	432	0 %	0	98 %	422	2 %	10	0 %	0	0 %	
1	113/1	Paint Sti Stringer	(LF)	4,041	0 %	0	89 %	3,601	10 %	400	1 %	40	0 %	
1	121/1	P/Stl Thru Truss/Bot	(LF)	1,050	0 %	0	90 %	950	0 %	0	10 %	100	0%	C
1	126/1	P/Stl Thru Truss/Top	(LF)	1,050	0 %	0	97 %	1,023	0 %	1	2 %	26	0 %	
- 1	131/1	Paint Sti Deck Truss	(LE)	944	0.0/		02.9/	970	C 0/	54	2.0/	20	0.0/	

Structure Inventory and Appraisal Sheet (English Units)

Str Unit	Elm/Env	Description	Units	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5
1	152/1	Paint Stl Floor Beam	(LF)	1,288	0 %	C	71 %	920	29 %	368	0 %	q	0 %	0
1	205/1	R/Conc Column	(EA)	14	100 %	14	0 %	0	0 %	9	0 %	q	0 %	۵
1	215/1	R/Conc Abutment	(LF)	108	0 %	0	100 %	108	0 %	0	0 %	d	0 %	O
1	234/1	R/Conc Cap	(LF)	210	2 %	4	90 %	190	5 %	10	3 %	6	0 %	O
1	300/1	Strip Seal Exp Joint	(LF)	24	100 %	24	0 %	0	0 %	0	0 %	q	0%	a
1	301/1	Pourable Joint Seal	(LF)	96	100 %	96	0 %	0	0 %	q	0 %	O	0%	0
1	302/1	Compressn Joint Seal	(LF)	120	50 %	60	50 %	60	0 %	Q	0 %	0	0 %	0
1	310/1	Elastomeric Bearing	(EA)	6	100 %	6	0%	0	0 %	q	0 %	0	0 %	0
1	311/1	Moveable Bearing	(EA)	6	67 %	4	33 %	2	0 %	q	0 %	Q	0 %	0
1	313/1	Fixed Bearing	(EA)	12	0 %	0	100 %	12	0 %	q	0 %	0	0%	0
1	331/1	Conc Bridge Railing	(LF)	1,134	1 %	10	44 %	497	46 %	527	9 %	100	0%	0
1	334/1	Metal Rail Coated	(LF)	1,156	0 %	0	100 %	1,156	0 %	q	0 %	o	0 %	0
1 1 -	358/1 -	Deck Cracking SmFlag	(EA)	- 1	0 %	0	100 %	- 1	- 0%	·d	- '0 %	O		· · · u
1	359/1	Soffit Smart Flag	(EA)	1	0 %	0	0%	0	100 %	1	0 %	q	0 %	0
1	363/1	Section Loss SmFlag	(EA)	1	0 %	0	100 %	1	0 %	q	0 %	q	0 %	O
1	503/1	Curbs	(LF)	2,210	0%	0	93 %	2,055	5 %	100	2 %	55	0 %	0
1	601/1	MisAlign/ot of plane	(EA)	1	100 %	_ 1	0 %	0	0 %	q	0 %	o	0 %	0
1	604/1	2nd Elem Dist	(EA)	. 1	100 %	1	0 %	0	0 %	q	0 %	q	0 %	0
1	606/1	Drains	(EA)	1	0 %	0	100 %	1	0 %	q	0 %	0	0 %	0

Str Unit	Elm/Env	Description	Element Notes
1	22/1	Concrete Deck - Protected w/ Rigi	The deck is wom.
1	110/1	Reinforced Conc Open Girder/Bea	The RCDG beams are worn and weathered. The left exterior beam in span 1 is cracking and spalling. The exterior ends at bearing are scaling and spalling. The right exterior beam in the last span is cracking and spalling on the bottom exposing steel near the pler
1	113/1	Painted Steel Stringer	The steet stringers at the floor beam connection at the joints are rusting and scaling with section loss of the stringer ends and connection plates. Some of the top flanges of the stringers where they are in connection with the concrete deck are rusting and scaling.
1	121/1	Painted Steel Bottom Chord Thru	There are areas of section loss and pack rust in the batton plates on the lower chords. There are several areas of section loss and pack rust of some of the lower chord splice plates and gusset plates.
1	126/1	Painted Steel Thru Truss (excl. bot	Impact damage has occurred to U1-L1 and U1-L2 on the north side of the smaller truss. Misalignment is sever on the compression member. There is also a crack in one of the flanges from the edge of the flange to the web. The tension member is twisted and out of alignment There is also some misalignment to a vertical on the right side of the larger truss. All the top lateral bracing on the truss has been impacted causing some misalignment.
1	131/1	Painted Steel Deck Truss	The horizontal bracing under the replaced sliding plate expansions are rusting and scaling with some small areas with 100% section loss in the flanges. The gusset and splice plates have some minor pack rust and some small areas of rusting and scaling with some section loss. The lower chords have some small areas of heavy rusting and scaling. In span 8 left at 3/4 point the lower chord has 90% section loss of the flanges of the channel beams and 30-40% section loss in the main web. This is the worst of the areas but there are several other areas similar.
1	152/1	Painted Steel Floor Beam	The floor beams under the joint and previous joints in the main trusses and deck trusses are rusting and scaling with some moderate section loss.
1	205/1	Reinforced Conc Column or Pile E	
1	215/1	Reinforced Conc Abutment	The abutments are worn and weathered with some minor deterioration.
1	234/1	Reinforced Conc Cap	The ends of pier caps 2 & 7 are deteriorating exposing reinforcing steel Pier cap 2 has about 2' of the end of the cap gone. Pier 6 cap has been rehabbed.
1	300/1	Strip Seal Expansion Joint	< none >
1	301/1	Pourable Joint Seal	< none >
1	302/1	Compression Joint Seal	The neoprene seals are beginning to deteriorate slightly and moving downward in the joints.
1	310/1	Elastomeric Bearing	< none >
1	311/1	Moveable Bearing (roller, sliding, e	The rockers are beginning to rust on the 2 main truss sections. The rockers were replace on the deck truss section with a thick neoprene pad.
1	313/1	Fixed Bearing	The fixed bearing are beginning to rust.
1	331/1	Reinforced Conc Bridge Railing	The concrete handrails have been rehabbed. Several of the post and rails have been replace. The remaining older rails and post are worn and weathered with some cracking.
1	334/1	Metal Bridge Railing - Coated	There are several areas of minor vehicular damage to the steel handrail on the truss. Surface rust is forming on the painted surface of the steel rail on the truss.
1	358/1	Deck Cracking	Unsealed transverse cracks exist in the deck.
1	359/1	Soffit of Concrete Deck or Slab	The bottom of the deck is cracking and spalling exposing some rusty reinforcing steel in various places. There are areas of map cracking and transverse cracking with some efflorescence present in some of the cracks.
1	363/1	Section Loss	See above discussions.
1	503/1		The concrete curbs are badly deteriorated in numerous places with rusting reinforcing steel exposed. Some of the areas have been natribed at

Structure Inventory and Appraisal Sheet (English Units)

			mitory and rippratour officer (English Citito)					
Str Unit	Elm/Env 601/1	Description Alignment, Out-of-Plane	Element Notes Span 8 (RCDG) has moved 2* westward.					
1	604/1	Second Element Distress	The concrete diaphragms of the RCDG are cracking and spalling around the exposed edges.					
1	606/1	Drains	50% of the drains are blocked.					
BRID	SE NOT	ES						
Fract	ure critic	cal Member location in Gene	eral Media					
PAST	INSPE	CTION						
Inspec	tion Da	te: 03/28/2013	Type: 3 Substandard (12 months)					
Inspec	tor:	DLARKIN	Pontis User Key: DLARKIN - Denny					
_			, and obs. Adj. Danielli Lamin					
Scope	: NBI:	Other:	☐ Element: ☒					
		rwater: Fracture						
	Onuc	iwater riacture	Onton.					
INSPE	CTION	NOTES						
Fracti team.	ure Criti	cal inspection was done on	October 4 2011, deck truss was inspected by snooper, thru truss was inspected by climb					
The e The k section There Lower	Deck truss was inspected with snooper, by Jonathon Beasley and Harry Greer. The east deck truss, second pier from east abutment has moderate to heavy rusting on bottom of gussett plate. The lower chord batten plates have moderate to heavy deterioration with rust, scaling, packrust and deformation present, along with section loss in several areas. There is heavy rust at the vertical bearing on the east end of decktruss. Lower chord downstream plating repairs have sloppy welds. Backend connection on lower chord downstream has mod to heavy deterioration and minor deformation. (see pics)							
<u> </u>	-	., , , , , ,	······································					
PAST	INSPE	CTION						
Inspec	tion Da	te: 03/21/2012	Type: 3 Substandard (12 months)					
Inspec	tor:	DLARKIN	Pontis User Key: DLARKIN - Denny					
Scope	NBI:	Other:	☐ Element: ☒ Critical: ☐					
INSPECTION NOTES								
Fracture Critical inspection was done on October 4 2011, deck truss was inspected by snooper, thru truss was inspected by climb team.								
Deck truss was inspected with snooper, by Jonathon Beasley and Harry Greer. The east deck truss, second pier from east abutment has moderate to heavy rusting on bottom of gussett plate. The lower chord batten plates have moderate to heavy deterioration with rust, scaling, packrust and deformation present, along with section loss in several areas. There is heavy rust at the vertical bearing on the east end of decktruss. Lower chord downstream plating repairs have sloppy welds.								